

Saltwater Intrusion Management with Conjunctive Use of Surface Water and Ground Water

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SYNOPSIS

Problem and Research Objectives

Due to the complexity of real aquifer systems and insufficiency of available data, we often encounter the situation that several simulation models agree satisfactorily to the same observation data. Nevertheless, these models can differ substantially from each other in model structure and in embedded model parameters. They can lead to substantially different predictions. This is the non-uniqueness problem in groundwater inverse modeling [Yeh, 1986]. Selection of a single "best" model is not sufficient when several competitive models are available. To take into consideration in model uncertainty, Bayesian model averaging (BMA) [Hoeting et al., 1999] was introduced to draw inferences (predictions) from multiple models. In BMA, model importance is represented by the posterior model probability, which is evaluated by the likelihood function and the prior model probability. The total prediction variance in BMA considers the within-model variance and the between-model variance. Evaluation of the likelihood function can be achieved by either the Monte Carlo (MC) simulation methods [Madigan and Raftery, 1994] or the Laplace approximation such that the model weights are calculated in terms of the Bayesian information criterion (BIC) [Raftery, 1995]. The latter approach is especially beneficial in groundwater modeling because the MC approach is usually too expensive for large-scale real-world groundwater models.

The objective of this study is to use the BMA technique to develop a Bayesian multi-model multi-parameterization (BMMMP) scheme to predict groundwater heads and evaluate head prediction uncertainty. In this study, we consider the uncertainty in the groundwater model as well as the uncertainty in the parameterization method to investigate the propagation of these uncertainties to the uncertainty of groundwater head predictions in the "1,500-foot" sand in East Baton Rouge Parish, Louisiana. The "1,500-foot" sand contains the Baton Rouge Fault, a distinct geological structure that restricts groundwater flow through the fault. According to different considerations on the Baton Rouge Fault characteristics, we develop three conceptual groundwater models: one model with a leaky fault, one model with an impermeable fault, and one model without a fault. For each groundwater model, we consider seven grain-size methods to estimate hydraulic conductivity at electrical resistivity-log sites. Different hydraulic conductivity distribution is then obtained through the GP method that combines the ordinary kriging (OK) method and the Voronoi tessellation (VT) method [Tsai 2006].

Methodology

(1) Bayesian Multi-Model Multi-Parameterization (BMMMP) Method

Consider a set of groundwater models denoted as $\{M^{(p)}, p=1, 2, \dots\}$, for groundwater simulation. In each groundwater model, $M^{(p)}$, a set of parameterization methods for estimating hydraulic conductivity is denoted as $\{\theta_q^{(p)}; q=1, 2, \dots\}$, where $\theta_q^{(p)}$ represents the parameterization method in groundwater model $M^{(p)}$. According to the law of total probability, the posterior probability of groundwater head predictions for given data \mathbf{D} , parameterization methods, and simulation models is

$$\begin{aligned} \Pr(\mathbf{h} | \mathbf{D}) &= E_M \left[E_\theta [\Pr(\mathbf{h} | M^{(p)}, \theta_q^{(p)}, \mathbf{D})] \right] \\ &= \sum_p \sum_q \Pr(\mathbf{h} | M^{(p)}, \theta_q^{(p)}, \mathbf{D}) \Pr(\theta_q^{(p)} | M^{(p)}, \mathbf{D}) \Pr(M^{(p)} | \mathbf{D}) \end{aligned} \quad (1)$$

where $\Pr(\mathbf{h} | \mathbf{M}^{(p)}, \theta_q^{(p)}, \mathbf{D})$ is the posterior probability of groundwater head predictions for given data, groundwater model $\mathbf{M}^{(p)}$ and parameterization method $\theta_q^{(p)}$. $\Pr(\theta^{(q)} | \mathbf{M}^{(p)}, \mathbf{D})$ is the posterior method probability of parameterization method $\theta_q^{(p)}$ for given data and groundwater model $\mathbf{M}^{(p)}$. $\Pr(\theta^{(q)} | \mathbf{M}^{(p)}, \mathbf{D})$ also represents the method weight. Consider the equal prior method probability. The posterior method probability can be approximated using the Bayesian information criterion (BIC):

$$\Pr(\theta^{(q)} | \mathbf{M}^{(p)}, \mathbf{D}) = \frac{\exp\left[-\frac{1}{2} \text{BIC}_q^{(p)}\right]}{\sum_j \exp\left[-\frac{1}{2} \text{BIC}_j^{(p)}\right]} \quad (2)$$

where

$$\text{BIC}_q^{(p)} = -2 \ln \Pr(\mathbf{D} | \mathbf{M}^{(p)}, \theta_q^{(p)}, \hat{\boldsymbol{\beta}}_q^{(p)}) + m_q^{(p)} \ln n \quad (3)$$

where $\hat{\boldsymbol{\beta}}_q^{(p)}$ are the maximum-likelihood (ML) estimated parameters in the method $\theta_q^{(p)}$, $m_q^{(p)}$ is the number of the parameters $\hat{\boldsymbol{\beta}}_q^{(p)}$, and n is the number of data \mathbf{D} . $\Pr(\mathbf{D} | \mathbf{M}^{(p)}, \theta_q^{(p)}, \hat{\boldsymbol{\beta}}_q^{(p)})$ is the likelihood function value of the heads for given model $\mathbf{M}^{(p)}$, method $\theta_q^{(p)}$. Considering the equal prior model probability, the posterior model probability $\Pr(\mathbf{M}^{(p)} | \mathbf{D})$ given the data is calculated through the Bayes rule:

$$\Pr(\mathbf{M}^{(p)} | \mathbf{D}) = \frac{\Pr(\mathbf{D} | \mathbf{M}^{(p)})}{\sum_i \Pr(\mathbf{D} | \mathbf{M}^{(i)})} \quad (4)$$

where $\Pr(\mathbf{D} | \mathbf{M}^{(p)}) = \sum_q \Pr(\mathbf{D} | \mathbf{M}^{(p)}, \theta^{(q)}) \Pr(\theta^{(q)} | \mathbf{M}^{(p)})$. $\Pr(\mathbf{M}^{(p)} | \mathbf{D})$ also represents the model weight. Using the law of total expectation, the means of the groundwater head predictions are

$$\begin{aligned} E(\mathbf{h} | \mathbf{D}) &= E_{\mathbf{M}} \left[E_{\theta} \left[E(\mathbf{h} | \mathbf{M}^{(p)}, \theta^{(q)}, \mathbf{D}) \right] \right] \\ &= \sum_p \sum_q E(\mathbf{h} | \mathbf{M}^{(p)}, \theta^{(q)}, \mathbf{D}) \Pr(\theta^{(q)} | \mathbf{M}^{(p)}, \mathbf{D}) \Pr(\mathbf{M}^{(p)} | \mathbf{D}) \end{aligned} \quad (5)$$

The total covariances of the groundwater head predictions are

$$\begin{aligned} \text{Cov}(\mathbf{h} | \mathbf{D}) &= E_{\mathbf{M}} E_{\theta} \left[\text{Cov}[\mathbf{h} | \mathbf{M}^{(p)}, \theta^{(q)}, \mathbf{D}] \right] + E_{\mathbf{M}} \text{Cov}_{\theta} \left[E[\mathbf{h} | \mathbf{M}^{(p)}, \theta^{(q)}, \mathbf{D}] \right] \\ &\quad + \text{Cov}_{\mathbf{M}} E_{\theta} \left[E[\mathbf{h} | \mathbf{M}^{(p)}, \theta^{(q)}, \mathbf{D}] \right] \end{aligned} \quad (6)$$

The means of heads can be approximated by $E[\mathbf{h} | \mathbf{M}^{(p)}, \theta^{(q)}, \mathbf{D}] = \mathbf{h}(\boldsymbol{\pi}_{GP,q}^{(p)})$. Using the linearization approach [Dettinger and Wilson, 1981; Tiedeman et al., 2003], the covariance matrix of heads is $\text{Cov}[\mathbf{h} | \mathbf{M}^{(p)}, \theta^{(q)}, \mathbf{D}] = \mathbf{J}_{\pi,q}^{(p)} [\text{Cov}_{GP,q}^{(p)}] [\mathbf{J}_{\pi,q}^{(p)}]^T$, where $\mathbf{J}_{\pi} = \partial \mathbf{h} / \partial \boldsymbol{\pi}|_{\pi_{GP}}$ is the Jacobian matrix and $[\text{Cov}_{GP}]$ is the covariance matrix when the GP method is used.

(2) Methodology Application to “1,500-foot” Sand, Baton Rouge, Louisiana

The methodology is applied to groundwater head prediction on January 1, 2020 in the “1,500-foot” sand in East Baton Rouge (EBR) Parish, Louisiana. The study area is shown in Figure 1.

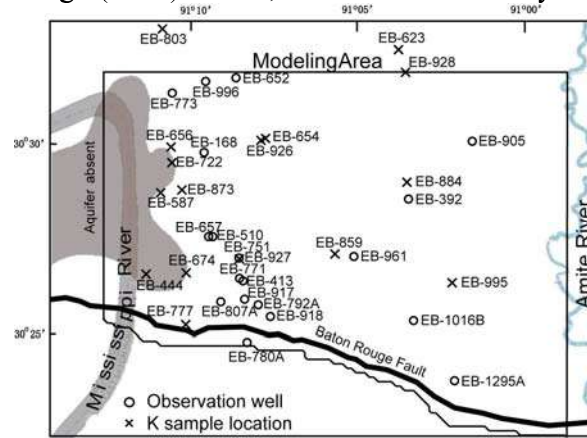


Figure 1. The study area: the “1,500-foot” sand.

The “1,500-foot” sand is one of the sand aquifers in Baton Rouge and is the major freshwater source to the public. The Baton Rouge Fault historically restricted saltwater south of the fault moving northward. However, groundwater levels in the East Baton Rouge Parish have declined by as much as 91 meters since the 1940's. The large cone of depression in the northern area of the Baton Rouge Fault has induced saltwater encroachment across the fault toward the pumping centers. In this study, we focus on groundwater head prediction using the BMMMP scheme. The study area in Figure 1 extends about 300 km² and includes a major part of the Baton Rouge metropolitan area.

To develop the regional groundwater model, we collected 706 groundwater observation records from 18 observation wells (see Figure. 1) for the period from January 1990 to December 2004 (15 years) from the USGS National Water Information System website. We also collected 21 electrical log data (see Figure 1) to determine the hydraulic conductivity and the aquifer thickness. The Capital Area Ground Water Conservation Commission provided monthly pumping data for 16 production wells that screen the “1,500-foot” sand. In this study, we developed a two-dimensional groundwater model using MODFLOW-2000 [Harbaugh et al., 2000]. The time-varied constant head boundary condition was used.

(3) Development of Three Groundwater Models and Seven Parameterization Methods

There is no direct information about the Baton Rouge Fault permeability that affects groundwater heads significantly. We adopted the Horizontal Flow Barrier [Hsieh and Freckleton, 1994] to estimate the fault hydraulic characteristic (HC), the hydraulic conductivity per unit width of fault. Using the observation data at EB-917 (north of the fault) and EB-780A (south of the fault), we estimated the hydraulic characteristic (HC) of the Baton Rouge Fault to be 0.000519 day⁻¹, which indicates a leaky fault with low permeability. For the comparing purpose, we built two additional groundwater models based on two extreme cases of the fault permeability. One is the impermeable-fault model, where the fault is impermeable. The other model is the no-fault model, where the fault is not considered in the model. Therefore, three groundwater models are:

- (1) Leaky-fault model (M_1)
- (2) Impermeable-fault model (M_2)
- (3) No-fault model (M_3)

To estimate the hydraulic conductivity in the study area, we used the electrical resistivity data from the 21 electrical resistivity wells obtained from Louisiana Water Science Center. Using Archie's law, we interpreted the resistivity reading into porosity. Then, we applied 7 grain-size empirical methods to calculate hydraulic conductivity at the E-log sites. The seven methods are listed in Table 1 [Kasenow, 2002].

Table 1. Seven grain-size methods to calculate the K value under the general expression of empirical formula, $K = b(g/\nu) f(n)d_e^2$, where $g = 9.8m/s^2$, $d_e = 0.22mm$, and the water kinematic viscosity, $\nu = 8.007 \times 10^{-7} m^2/s$ at $30^\circ C$.

Grain-size method	b	Function of porosity f(n)	Domain of applicability
Kozeny-Carman	1/180	$\frac{n^3}{(1-n)^2}$	Fine to large grain sands
Sauerbrei	3.75×10^{-3}	$\frac{n^3}{(1-n)^2}$	Sand and sandy clay
Slichter	0.01	$n^{3.287}$	Fine to large grain sands
Terzaghi	6.1×10^{-3}	$\left(\frac{n-0.13}{\sqrt[3]{1-n}} \right)^2$	Large-grain sands
Kruger	4.35×10^{-3}	$\frac{n^3}{(1-n)^2}$	Medium-grain sands
Zunker	1.2×10^{-3}	$\left(\frac{n}{1-n} \right)^2$	Fine and medium-grain sands
Zamarin	8.2×10^{-3}	$\frac{n^3}{(1-n)^2} (1.275 - 1.5n)^2$	Large-grain sands

Once the hydraulic conductivity values are determined by the grain-size methods at the E-log locations, we use the generalized parameterization (GP) method [Tsai, 2006] to estimate the spatially correlated log-hydraulic conductivity ($\pi = \ln \mathbf{K}$). The GP method in this study combines the ordinary kriging (OK) and Voronoi tessellation (VT), a zonation method. Therefore, seven GP methods are considered and denoted as (1) GP-Kozeny-Carman (θ_1), (2) GP-Sauerbrei (θ_2), (3) GP-Slichter (θ_3), (4) GP-Terzaghi (θ_4), (5) GP-Kruger (θ_5), (6) GP-Zunker (θ_6), and (7) GP-Zamarin (θ_7).

Principal Findings and Significance

(1) Estimation of Model Weights and Method Weights

The model weights and method weights play a very important role in the BMMMP because they represent the model and method importance. Using the BIC to calculate model weights in the

BMA reveals the model selection result using Occam's window [Raftery, 1995]. Occam's window determines if the model would be selected based on the log posterior ratio of the considered model against the best model. The problem of using Occam's window is the too narrow window size, which easily rejects good models. We developed a variance window, which defines the window size to accept models based on the variance of the error chi-squares [Tsai and Li, 2007]. In this study, we use a 5% significance level in Occam's window and two times of the standard deviation of chi-squares as the window size. The scaling factor is 0.0798.

Table 2 lists the weights of the seven GP methods in each groundwater model. The GP-Kozeny-Carman method is the best GP method in the leaky-fault model and impermeable-fault model while the GP-Sauerbrei method is the second best method. However, the GP-Slichter method is the single best method when the no-fault model is used.

Table 2. Posterior probabilities of GP methods (method weights) in groundwater models.

GP Methods	Posterior Method Probabilities in Leaky-Fault Model	Posterior Method Probabilities in Impermeable-Fault Model	Posterior Method Probabilities in No-Fault Model
GP-Kozeny-Carman	57.01%	73.48%	0.00%
GP-Sauerbrei	29.31%	23.40%	0.00%
GP-Slichter	0.00%	0.00%	100%
GP-Terzaghi	0.00%	0.00%	0.00%
GP-Kruger	6.29%	1.61%	0.00%
GP-Zunker	7.38%	1.51%	0.00%
GP-Zamarin	0.00%	0.00%	0.00%

Table 3 lists the model weights for three groundwater models. The leaky-fault model is the best model with a weight of 67.01%. The impermeable-fault model gains about one third of the total weight. The no-fault model is rejected.

Table 3. Posterior probabilities of groundwater models (model weights).

	Leaky-Fault Model	Impermeable-Fault Model	No-Fault Model
Posterior Model Probabilities	67.01%	32.99%	0.00%

Figure 2(a) shows that the BMMMP is able to fit well the head observations at EB-168, which are bounded by the one-standard deviation bounds of the BMMMP. Figure 2(b) demonstrates that the leaky-fault model and impermeable-fault model are good models. The no-fault model is unacceptable.

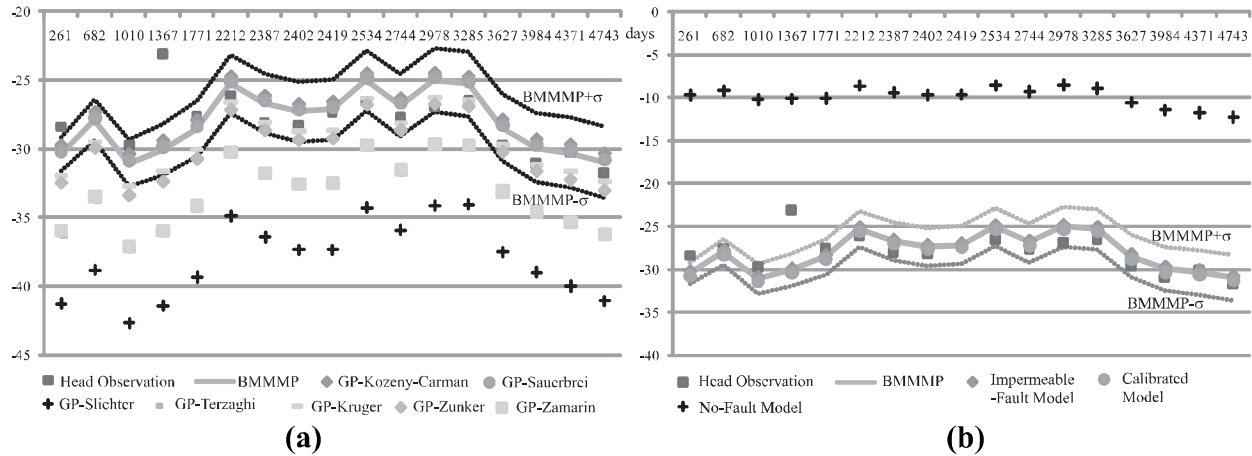


Figure 2: Comparisons to the observed groundwater heads at observation well EB-168 (a) BMMMP against individual GP methods using the leaky-fault model, and (b) BMMMP against individual groundwater models.

(2) Head Predictions Using BMMMP

We predicted the groundwater head on January 1, 2020 by using the monthly averaged pumping rate and head boundary conditions in the three years (2002-2004). In Figure 3, we compared the head predictions on January 1, 2020 using the best GP method (GP-Kozeny-Carman) in the leaky-fault model against that using the BMMMP. Because the GP-Kozeny-Carman has more than 50% of the total weight, the BMMMP and the best single model result in similar predicted groundwater heads.

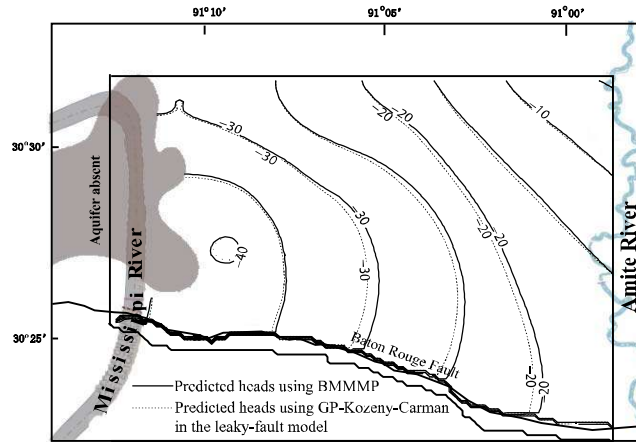


Figure 3: Predicted groundwater heads on January 1, 2020.

The variances of head predictions on January 1, 2020 using the BMMMP is shown in Figure 4, which include the within-method variances, between-method variances, between-model variances, and total variances. The large head prediction variances come from the GP methods. The between-method variances are small because the GP-Kozeny-Carman dominates in both leaky-fault model and impermeable-fault model. The between-model variances are slightly higher than the between-method variances. The head prediction variances increase toward the middle-east area near the fault due to less hydraulic conductivity samples and fewer head

observations in this area. More K measurements and head observations can significantly reduce the prediction uncertainty in this area.

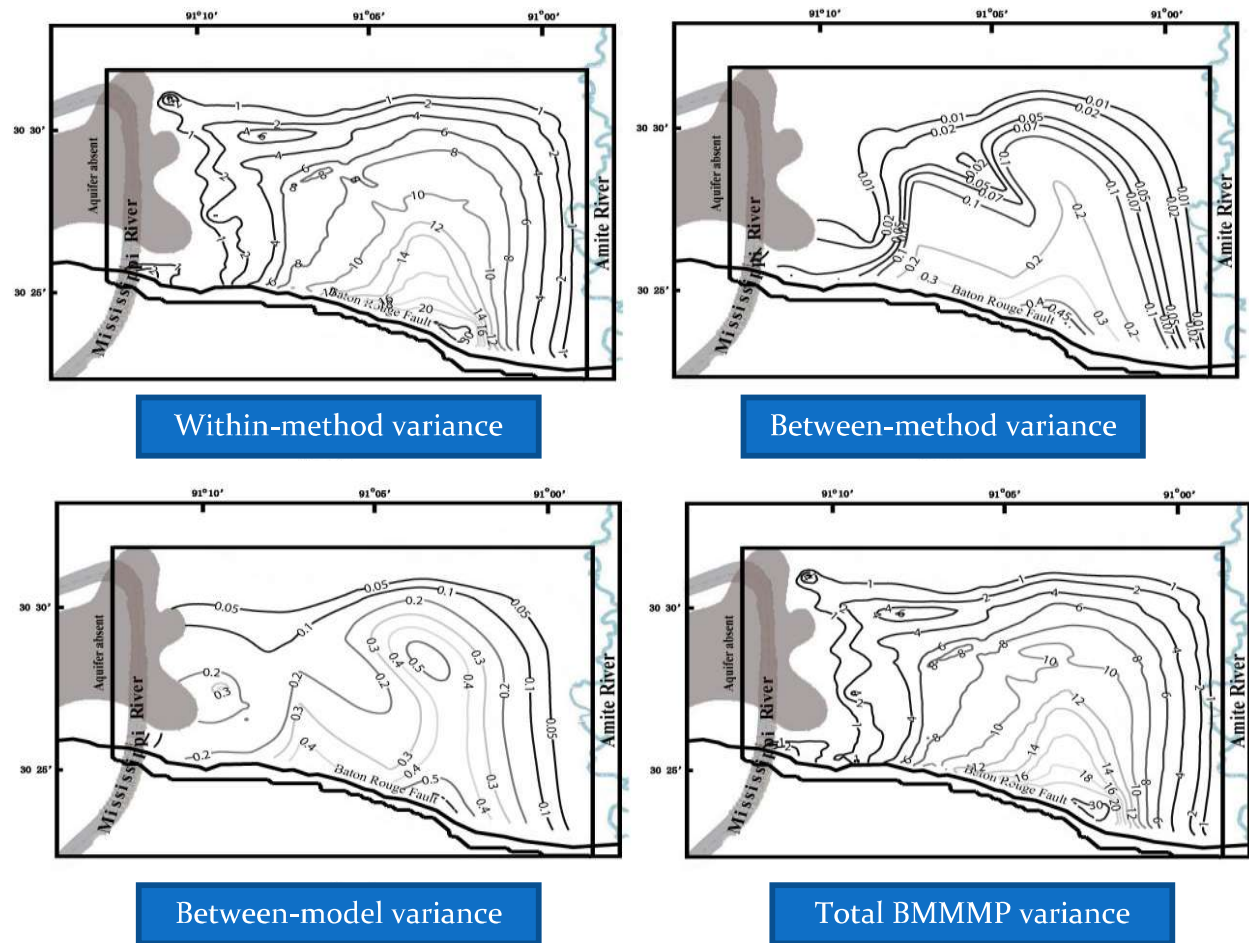


Figure 4: Head prediction variances.

In conclusion, the BMMMP scheme provides a rigorous approach to estimate the head predictions and to evaluate prediction uncertainty by incorporating multiple groundwater models and multiple parameterization methods. This approach can avoid overconfidence in using a single method and a single simulation model and gain more trust in the predicted results.

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